

Efficacy of FRP Hooping in Masonry Domes: A Simple Numerical Approach [†]

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Abstract: A simple numerical approach to predict the efficacy of FRP hooping in historical masonry domes is presented. The dome is modelled with 8-noded elastic hexahedron elements connected by 1D trusses/springs on meridians and on parallels, where all the non-linearity takes place. The aim is to simulate the nonlinear behaviour of domes through every FE commercial software equipped only with non-linear 1D elements, namely point contacts and cutoff bars. The constitutive behaviour of the trusses is assumed to be either perfectly brittle or perfectly ductile. A possible orthotropic behaviour and the no-tension material case can be reproduced. External retrofitting is simulated using trusses with an elastic perfectly ductile behaviour, assuming a perfect bond between the substrate and the reinforcement and imposing an ultimate strength for the trusses, which takes into account the possible debonding/delamination from the substrate in a conventional way. The Italian code CNR DT200 and the existing specialized literature are used as references. The models are benchmarked on a masonry dome reinforced with three hooping FRP strips and experimentally tested at the University Architecture Institute of Venice IUAV, Italy. The procedure is validated through extensive comparisons with available experimental data and numerical results obtained in the literature with a variety of different models. Through the extensive comparisons that were made and discussed, the robustness and simplicity of the procedure are proven.

Keywords: FRP; hoop reinforcement; masonry domes; non-linearity; collapse load; FEM; point contact; plastic hinge; cutoff bars



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1. Introduction

The verification methods presented here aim at determining the collapse load of a hemispherical masonry dome subjected to an axisymmetric load on the crown. The same method is considered for the reinforcement of historical or overloaded domical structures by means of FRP strips.

The case study is a physical model with the same characteristics (about 2.2 m in diameter, 0.12 m thick with 0.20 m wide oculus on the top) and mechanical properties as detailed in [1]. It was built and tested under a vertical load and in case of reinforcement until collapse in the laboratory of the IUAV in Venice [2]. From these few mechanical tests, a series of numerical and analytical computations [1,3–9] have been performed to find the ultimate collapse load and the position of plastic hinges.

The method finds its significance and utility for both research and professional works. Indeed, all of the methods that were found in the literature are, in the abovementioned cases, expensive in terms of time and computational capacity or neglect important features of masonry [10], while the proposal presented here is quick, it can be implemented in every simple finite-elements (FE) software and considers masonry tensile resistance and orthotropy. The method has been validated by the authors in precedent works on arches [11] and domes [12].

In the paragraphs below, the reader may find brief information about two ways of modelling for nonlinear analysis (involving brittle and ductile elements) and the consequent results. Finally, the second method of elastic perfectly ductile elements is applied to simulate the behaviour of FRP strips and demonstrate their efficacy.

2. Modelling Methods

The modelling and the nonlinear analysis have been performed in a commercial FE environment. A meridian slice of a hemispherical dome (the reader may find an example in Figure 1) was used to speed up computations and, finally, the results were made commensurate to the whole dome using simple math.

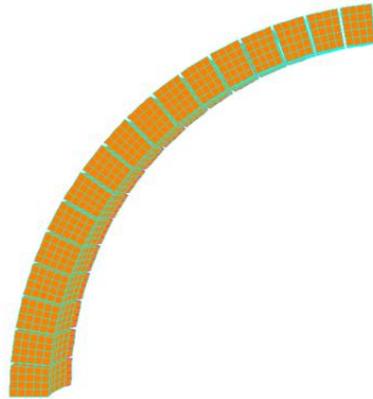


Figure 1. Initial undeformed meridian slice of the hemispherical dome reported as an example.

To replicate the axisymmetric load applied to the physical model, a point load on the upper crown, close to the oculus was set by imposing a unit displacement.

Considering that failures in such structures often occur at the mortar joints and not in the clay bricks, these have been modelled as elastic hexahedral elements (3D) and are never expected to fail. As masonry domes show material and geometrical nonlinearities, these have been lumped in the mortar joints, modelled by 1D finite elements—already implemented in the software used, namely Point Contacts (PCs) and Cutoff Bars (CoBs). The next two paragraphs detail and explain the choice.

2.1. Elastic Perfectly Brittle Joints: Point Contact

In the first way of modelling, joint nonlinearity is set by elastic perfectly brittle PCs, which are categorised as 1D “beam elements” and used under Heyman’s hypotheses of no-tension materials and the small displacements hypothesis. They are set to work in compression only. No sliding is accounted for.

Results

The results of such modelling are shown in the sensitivity analysis in Figure 1. It compares the collapse loads resulting from small changes in the tensile resistance assigned to horizontal joints only ($f_{T,h}$), while vertical joints were set to be no-tension. The curves show some mutual differences in their behaviour in the first steps of the simulation (rectangle a in Figure 2), with a progressive loss of structural stiffness and the development of plastic hinges. While in the steps near the end, they tend toward the asymptote value given by the lower-bound Limit Analysis (LA) found in [9] (rectangle b in Figure 2).

In spite of a largely conservative collapse load value, such an analysis gives an accurate position of the plastic hinge (at 45° from the vertical axis), visible in the deformed shape in Figure 3.

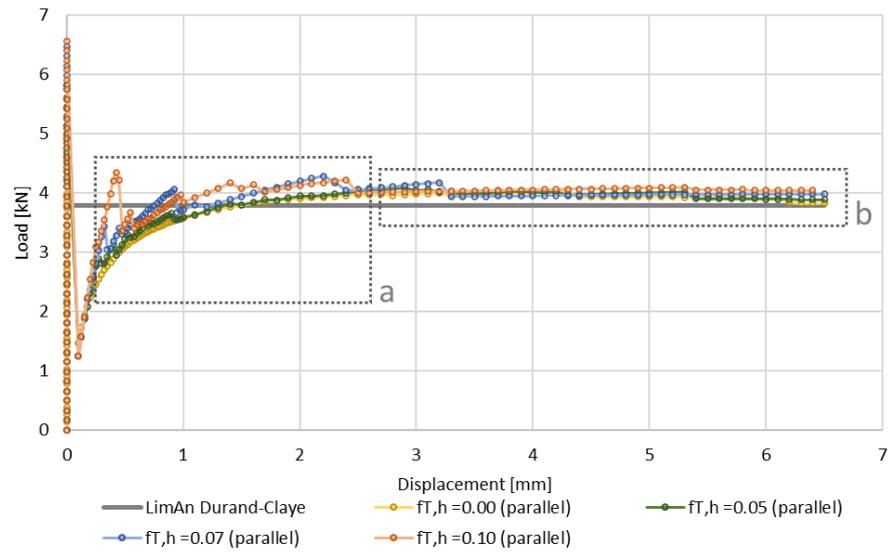


Figure 2. Sensitivity analysis for the no-tension material hypothesis model (brittle PC). (a) sawtooth profile showing a progressive loss of structural strength; (b) asymptotic behaviour of load bearing capacity curves.

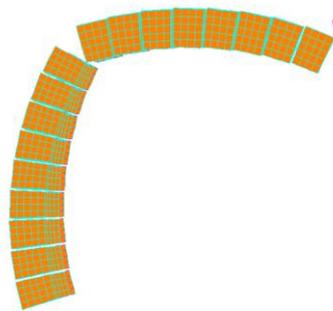


Figure 3. Formation of the intermediate plastic hinge at about 45° from the vertical axis. Front view of the meridian slice.

2.2. Elastic Perfectly Ductile Joints: Cutoff Bars

The second type of model sees nonlinear joints configured as elastic perfectly ductile truss elements (1D) [11] with predefined tensile and compressive strengths, namely CoBs. The cutoff values for tension and compression are suitably tuned to simulate masonry orthotropy.

Additionally, a rigid base larger than that of the point load (a scheme in Figure 4a) is added to simulate the load distribution of a real architectural element on the crown of the dome. Then, to prevent out-of-plane sliding, shear resistance has been provided by a complex joint construction involving rigid beams, CoBs and shear trusses (the reader may refer to Figure 4b for more clarity). The failure of this structure is explained by the formation of a plastic hinge. For the sake of brevity, no more information will be reported here.

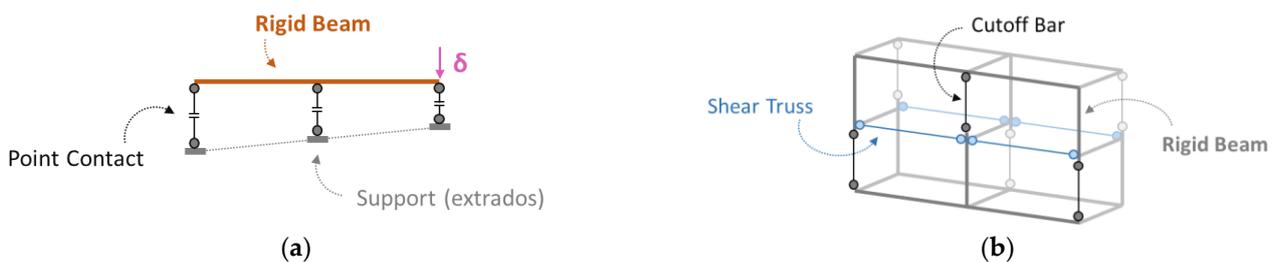


Figure 4. (a) Scheme of the load pattern for load distribution; (b) scheme of the shear resisting joint.

Results

The results of the second model are shown in the essential sensitivity analysis of Figure 5. Each curve results from tuning the tensile resistance values along horizontal ($f_{T,parallel}$) and vertical ($f_{T,meridian}$) directions. The values, collected in Table 1, are always multiplied by the influence area of the joint, which depend on the position along the meridian.

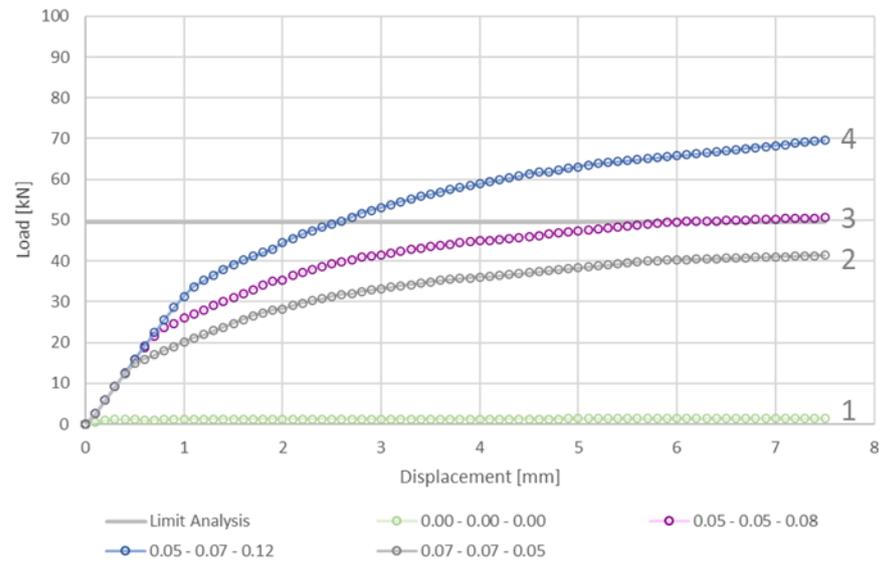


Figure 5. Essential sensitivity analysis accounting orthotropy (ductile CoBs).

Table 1. Data for an essential sensitivity analysis.

	$f_{T,parallel}$ [MPa]		$f_{T,meridian}$ [MPa]
	Upper	Haunch	
1	0.00	0.00	0.00
2	0.07	0.07	0.05
3	0.05	0.05	0.08
4	0.05	0.07	0.12

In Figure 5, it can be noted that a sensible increase in the load bearing capacity can be achieved by acting on the $f_{T,meridian}$. Comparison with the no-tension material case (curve 1) shows a far higher capacity when tensile resistances are accounted for. The other curves represent significant cases of the tuning of variables considering orthotropy and leading to correspondence with the literature above the lower-bound LA [7] (Curve 3).

In Figure 6 the deformed shape of the meridian slice is shown. As can be seen, there is no evident position of an intermediate plastic hinge, hence it is said to be “smeared”.

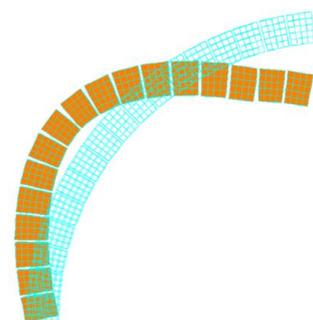


Figure 6. Formation of a smeared intermediate plastic hinge. Front view of the meridian slice.

The chart in Figure 7 shows the validation of the best result from the sensitivity analysis against those coming from the FE [1,3,4,7,8] approaches from literature.

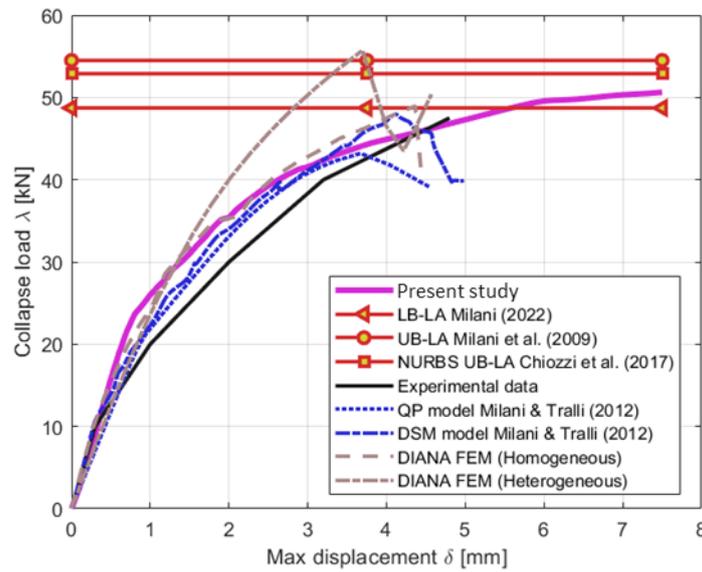


Figure 7. Validation of the results by comparison with results from the literature obtained by the DE and FE methods and analytical calculations [2,7–9].

2.3. Elastic Perfectly Ductile Cutoff Bars Applied to FRP Hoop Reinforcement

In order to prove the efficacy of applying hoop reinforcement on the extrados of domes, FRP strips have been simulated by the addition of elastic perfectly ductile CoBs [11–16] with mechanical parameters and relative positions derived from [1,17,17].

The following chart in Figure 8 contains the collapse load computed by LA and DEM [1]. As the reader may note, the agreement between final results is satisfying enough. The area in between the DEM curve (DIANA model, blue dotted line) and the FEM curve resulting from this method is due to a different way of modelling (more discretised nonlinearity in the DIANA model [1]). Compared to the upper-bound LA, the FE model is shown to be more conservative but proves that FRP strips increase the collapse load of a dome by preventing the formation of meridian cracks.

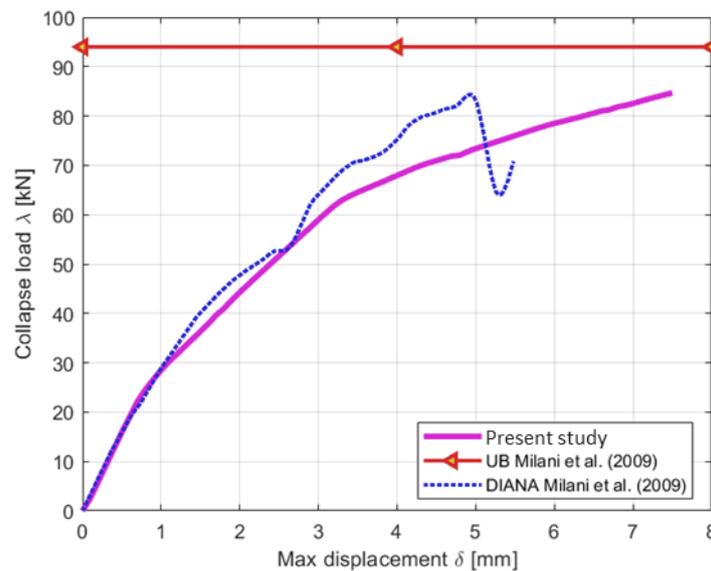


Figure 8. Collapse load numerically computed after the addition of ductile CoBs representing the application of FRP strips [1].

The following Figure 9 indicates where the FRP strips exert their confinement action against the formation of meridian cracks and an annular hinge. By comparison between Figures 6 and 9, the efficacy of FRP reinforcement is shown.

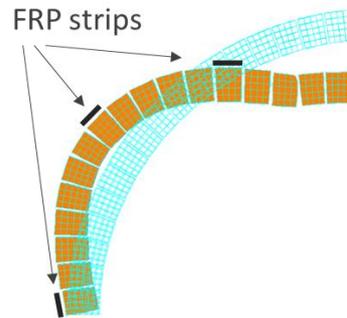


Figure 9. Deformed shape of the reinforced case. Side view of the meridian.

3. Conclusions

To recap the results that were obtained, the first method models masonry nonlinearity using elastic perfectly brittle PC, neglecting most of masonry's tensile resistance (by Heyman's hypotheses). Whereas in the second method, masonry nonlinearity is expressed by elastic perfectly plastic CoBs. The brittle behaviour of the first model ends in providing an accurate position of the plastic hinge but also a very conservative value in terms of collapse load. The contrary is achieved by the ductility imposed in the second model. This one was used as a baseline for the application of FRP strips, again by the addition of ductile CoBs. Indeed, the collapse load computed for the reinforced case (Figure 8) was almost twice that of the unreinforced case (Figure 9), demonstrating the efficacy of such interventions. The method itself proved to be robust and is therefore a very useful and practical way to model such additions for retrofitting or safety purposes.

Despite the simplicity of the modelling and the short computational time, the present method can be further implemented with automatic choices depending on some initial data to be input (from standards or surveys).

The methods can be applied to real masonry hemispherical domes, both in unreinforced and reinforced cases. They have both proven to be numerically stable and robust; useful for the purpose of verification (the first in finding the intermediate plastic hinge position, and the second and third in finding the collapse load); and to be trustworthy (by comparison with the analytical and numerical results). Hence, they may easily be used in professional life in parallel or in substitution of LA.

Supplementary Materials: The presentation materials can be downloaded at: <https://www.mdpi.com/article/10.3390/IOCBD2023-15936/s1>.

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